

American Institute of Timber Construction



MEETING BOOK

PANEL OF DIRECTORS

OF THE

AITC STANDARD COMMITTEE

DECEMBER 18, 2009

Confidential - Not for Publication or Reproduction



American Institute of Timber Construction

7012 South Revere Parkway • Suite 140 • Centennial, CO 80112

Phone: 303/792-9559 Fax: 303/792-0669

Panel of Directors of the AITC Standards Committee

Meeting Agenda

Disposition of Views and Objections

December 18, 2009

Time: TBA

- I. Call to order and introductions.
- II. Review of procedure and agenda.
- III. Consideration of New Members.
- IV. Disposition of Views and Objections.
- V. Other Business.
- VI. Adjournment.



AITC Standards Committee

Standard Specifications for Structural Glued Laminated Timber of Softwood Species (AITC 117)

Secretariate: AITC

Chairman: Jeffrey Linville

Vice-Chairman: Marc Mullins

Ballot 1: Summary of Votes

				Ballot 1				
				Aff	Aff w/Com	Neg	Abs	
1	Beineke	Larry	PFS Corporation		XXX			General Interest
2	Bevard	Bruce	Cascade Structural Sales, LLC	XXX				General Interest
3	Bonfield	Dochka	None					User
4	Brown	James	Engineering Consultants, Inc		XXX			User
5	Burley	Gary	Unit Structures, LLC	XXX				Manufacturer
6	Caskey	Gary	EnWood Structures, LLC					Manufacturer
7	Cheung	Kevin	Western Wood Products Association		XXX			Producer
8	Dausman	Alan	Bechtel	XXX				User
9	Devisser	Don	West Coast Lumber Inspection Bureau			XXX		Producer
10	Dexter	Ryan	Wood Truss Council of America					General Interest
11	Douglas	Brad	AF&PA	XXX				Manufacturer
12	Drake	Kerlin	Anthony Forest Products					Manufacturer
13	Dunn	Gary	Boise Cascade					Manufacturer
14	Fiorella	Jeff	Boozer Laminated Beam Co., Inc.	XXX				Manufacturer
15	Forman	John	Alamco Wood Products	XXX				Producer
16	Gareis	Bill	Ashland	XXX				Manufacturer
17	Gilham	Paul	Western Wood Structures, Inc.	XXX				User
18	Griswold	Jim	Filler King Company					Manufacturer
19	Howard	Jon	Rosboro Lumber Company		XXX			Manufacturer
20	Hucke	Doug	Timberweld			XXX		Producer
21	Jennings	Jessica	Georgia-Pacific Resin, Inc.				XXX	Manufacturer
22	Jones	Ed	Laminated Timber, Inc.	XXX				User
23	Kumar	Jeet	U.S. Department of Veterans Affairs	XXX				Manufacturer
24	Lane	Mike	QB Corporation					User
25	Leatherman	Brent	Timber Tech Engineering, Inc	XXX				Producer
26	Linville	Jeffrey	AITC		XXX			Manufacturer
27	Mackey	Joel	Rigidply Rafters, Inc.					Manufacturer
28	McCormack	Peter	Log Services, Inc.	XXX				User
29	Montgomery	Donald	Montgomery Consulting LLC	XXX				User
30	Mullins	Marc	APA - The Engineered Wood Association		XXX			Manufacturer
31	Pearson	Victor	American Laminators	XXX				Manufacturer
32	Poindexter	J.P.	Hexion	XXX				Producer
33	Pooley	Bruce	None	XXX				User
34	Rhude	Maurice	Sentinel Structures, Inc					Manufacturer
35	Robak	Glen	Weyerhaeuser					User
36	Silva	Richard	National Park Service				XXX	General Interest
37	Stochlia	Kurt	ICC-ES		XXX			General Interest
38	Story	Richard	Enterprise Engineering Consultants, Ltd.		XXX			User
39	Tolley	Brian	Casco Adhesives		XXX			Producer
40	Trott	Philip	Canadian Wood Council					Manufacturer
41	VanCott	Craig	Unadilla Laminated Products				XXX	Manufacturer
42	Wendler	Steve	Dynea USA	XXX				Producer
43	Whelan	Chris	Purbond	XXX				Producer
44	White	Don	National Casein of California	XXX				Producer
45	Whittle	Carlton	Structural Wood Systems					Manufacturer
				19	9	2	3	33
				58%	27%	6%	9%	

Ballot Passes
Majority of Committee has voted

mfg	pro	user	GI
			Aff w/Com
			Aff
		Aff w/Com	
Aff			
	Aff w/Com		
		Aff	
	Neg		
Aff			
Aff			
	Aff		
Aff			
		Aff	
Aff w/Com			
	Neg		
		Aff	
Aff			
	Aff		
Aff w/Com			
		Aff	
		Aff	
Aff w/Com			
Aff			
	Aff		
		Aff	
			Aff w/Com
		Aff w/Com	
	Aff w/Com		
	Aff		
	Aff		
	Aff		

Manufacturer		
Aff	Aff w/Com	Neg
6	3	0
100%		0%

Producer		
Aff	Aff w/Com	Neg
6	2	2
80%		20%

User		
Aff	Aff w/Com	Neg
6	2	0
100%		0%

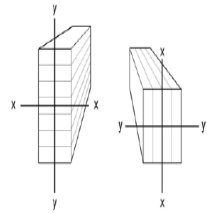
General Interest		
1	2	0
100%		0%

**Affirmative
With
Comment**

Panel of Directors of the AITC Standards Committee

Affirmative with Comment Votes

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 1 1.2	Kevin Cheung	Species – “ES” is the official species mark for “Engelmann Spruce” (see Standard Grading Rules for Western Lumber). The use of “ES” for “Eastern Spruce” in AITC 117 will create confusion in the marketplace.	Species Group – Eastern Spruce Symbol – ES Species that may be included in the group – Black Spruce, Red Spruce, White Spruce	Species – “ES” is the official species mark for “Engelmann Spruce” (see Standard Grading Rules for Western Lumber). The use of “ES” for “Eastern Spruce” in AITC 117 will create confusion in the marketplace.	No action recommended. Historically, ES has been used as Eastern Spruce in this standard dating back to AITC 117-1987.
Item 2 1.2	Brian Tolley	I don’t understand the need for the ES (Eastern Spruce) category when all 3 species are grouped in both SW (Softwood) and SFP (Spruce Pine Fir). Wouldn’t this triple classification just add confusion to designers? Should any species be in multiple groupings?	Species Group – Eastern Spruce Symbol – ES Species that may be included in the group – Black Spruce, Red Spruce, White Spruce	Remove the species grouping or remove those species from the other 2 classifications. Make sure each species is represented in only one species group.	No action recommended.
Item 3 1.5	James Brown	Add “Allowable” in two places.	Timbers with symmetric lay-ups are referred to as “balanced” and have the same design values for positive and negative bending. Timbers with asymmetric lay-ups are referred to as “unbalanced” and have higher design stresses for positive bending than for negative bending.	Change 3 rd line to read: “and have the same allowable design values---.” Change 4 th line to read: “to as “unbalanced” and have higher allowable design stresses---.”	Accept changes as editorial.

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 4 1.7	Marc Mullins	The last paragraph still references “Western Species.” I thought the intent of this standard is to avoid using such a term	Southern Pine laminations are typically surfaced to 1-3/8 in. (35 mm) thick, and Western Species laminations are typically surfaced to 1-1/2 in. (38 mm) thick. Laminations 3/4 in. (19 mm) thick are often used for curved members of both Southern Pine and Western Species. Depths matching standard I-joist depths are also available from many manufacturers.	Replace it by “laminations of other softwood species.”	Accept changes as editorial.
Item 5 1.8	Doug Hucke	Poorly worded	The minimum radius shall not be less than...	“the radius shall not be less than...” or “the minimum radius shall be...”	Accept 1 st proposed wording as editorial.
Item 6 1.9	Marc Mullins	The use of “Appearance Grades” may be confusing with the mechanical property requirements.	Appearance grades shall be specified in accordance with ANSI/AITC A190.1 (5) or as agreed upon between buyer and seller. The reference design values are independent of the appearance grades.	Suggest changing it to “Appearance Classifications,” as used by APA Y110.	No action recommended. The term “Appearance Grades” is used throughout ANSI/AITC A190.1-2007.
Item 7 2.1	Jeff Linville	Add a drawing showing x-x and y-y axes.	...causing bending about the x-x axis. Values designated with a subscript “y” are based on transverse loads applied parallel to the wide faces of the laminations, causing bending about the y-y axis.	Design values for structural glued laminated timber are dependent on the orientation of the member relative to the applied loads. Values designated with a subscript “x” are based on transverse loads applied perpendicular to the wide faces of the laminations, causing bending about the x-x axis (Figure 2.1-1). Values designated with a subscript “y” are based on transverse loads applied parallel to the wide faces of the laminations, causing bending about the y-y axis (Figure 2.1-1). 	Accept changes as editorial.

Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 8 2.4	Larry Beineke	Reference to AITC Technical Note 18 (3) should not be deleted. It is still in the “References” and presumably still provides guidance in the evaluation of the effects of checks and splits.	The tabulated shear design values permit minor amounts of checking ($\leq 15\%$ of beam width) without explicit consideration by the designer. Accredited Inspection and Testing Agencies typically provide guidelines for the analysis of severely checked beams.	Add to the end of the fourth paragraph of the section, “See AITC Technical Note 18 (3) for additional assistance.”	No action recommended.
Item 9 2.4	Rick Story	Add comment “Also See AITC Technical Note 18.” to end of last paragraph	The tabulated shear design values permit minor amounts of checking ($\leq 15\%$ of beam width) without explicit consideration by the designer. Accredited Inspection and Testing Agencies typically provide guidelines for the analysis of severely checked beams.	Add comment “Also See AITC Technical Note 18.” to end of last paragraph	No action recommended.
Item 10 2.4	Kurt Stochlia	The last paragraph notes “Accredited Inspection and Testing Agencies”. There are several definitions and requirements that could apply to these agencies. It might be helpful to provide a clarification as to the intent.	The tabulated shear design values permit minor amounts of checking ($\leq 15\%$ of beam width) without explicit consideration by the designer. Accredited Inspection and Testing Agencies typically provide guidelines for the analysis of severely checked beams.	Since there are several ways to resolve this, I am not proposing any specific solution since I am not sure of your intent.	Editorially change to “Accredited Inspection Agency” as this term is defined by IAS and is used throughout ANSI/AITC A190.1-2007 and AITC 405 (ANSI).
Item 11 2.5	Marc Mullins	The reference to “2006 NDS (6) and later” is not appropriate for a national consensus standard.	...stability calculations for columns and beams (2005 NDS [®] (6) and later).	Suggest deleting “and later.”	Accept change and re-circulate as substantive.

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 12 2.5	Marc Mullins	In the paragraph after Equation 1.2.4, it states that “For span to depth ratios of 15 or less, deflections due to shear stresses should be considered.” The ratio of 15 is inconsistent with Section 8.9 of ASTM D3737.	For span to depth ratios of 15 or less, deflections due to shear stresses should be considered.	Suggest the ratio be changed to “less than 14” to avoid confusion with ASTM D3737.	Accept change and re-circulate as substantive.
Item 13 2.9	Marc Mullins	The section is confusing.	<p>For Southern Pine, the design value for radial tension (tension perpendicular to the longitudinal axis of a curved member), F_{rt}, shall be equal to 1/3 of the shear design value, F_{vx}, for non-prismatic members. Radial reinforcement shall not be required.</p> <p>For all other softwood species, the reference design value for radial tension shall be limited to 15 psi (100 kPa) for loads other than wind or earthquake loads. If the calculated radial tension stress (due to loads or load combinations not including wind or seismic loads) exceeds 15 psi (100 kPa) multiplied by appropriate adjustment factors, radial reinforcement shall be required. Design values for radial tension for radially-reinforced members shall be limited to 1/3 of the shear design value for non-prismatic members. Radial reinforcement shall be designed in accordance with the Timber Construction Manual (4). For wind and earthquake loading, the design value for radial tension shall be 1/3 of the shear design value for non-prismatic members.</p>	Suggest tabulate the radial tension values as follows: See Table Below.	Add table.

<u>Loading type</u>	<u>Softwood species other than Southern pine</u>	<u>Southern pine</u>
<u>Wind or seismic</u>	<u>1/3 of F_{vx} for non-prismatic members</u>	<u>1/3 of F_{vx} for non-prismatic members</u>
<u>Other loading</u>	<u>15 psi^(a)</u>	<u>1/3 of F_{vx} for non-prismatic members</u>

^(a) If the calculated radial tension stress (due to loads or load combinations not including wind or seismic loads) exceeds 15 psi (100 kPa) multiplied by appropriate adjustment factors, radial reinforcement shall be required. Design values for radial tension for radially-reinforced members shall be limited to 1/3 of the shear design value for non-prismatic members. Radial reinforcement shall be designed in accordance with the Timber Construction Manual (4).

Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 14 2.12	Jeff Linville	In Section 2.1, we defined x-x axis and y-y axis. We should use this language here, rather than “loads applied perpendicular to the wide faces of the lamination” and “loads applied parallel to the wide faces of the laminations,” which can be confusing.	<p>The design values in Table A1 and Table A1-Expanded are applicable to members with 4 or more laminations and are intended primarily for members stressed in bending due to loads applied perpendicular to the wide faces of the laminations. Design values are included, however, for axial loads and bending loads applied parallel to the wide faces of the laminations. The values in Table A1 are for the industry recommended stress classes. Each stress class is representative of a group of combinations with similar design values. Design values for individual combinations are shown in Table A1-Expanded.</p> <p>Table A2 contains design values for timbers with uniform grade lay-ups. These combinations are intended primarily for timbers loaded axially or in bending due to loads applied parallel to the wide faces of the laminations. Design values are included, however, for bending due to loads applied perpendicular to the wide face of the laminations.</p>	<p>The design values in Table A1 and Table A1-Expanded are applicable to members with 4 or more laminations and are intended primarily for members stressed in bending due to loads applied perpendicular to the wide faces of the laminations. <u>about the x-x axis.</u> Design values are included, however, for axial loads stresses and stresses from bending loads applied parallel to the wide faces of the laminations <u>about the y-y axis.</u> The values in Table A1 are for the industry recommended stress classes. Each stress class is representative of a group of combinations with similar design values. Design values for individual combinations are shown in Table A1-Expanded.</p> <p>Table A2 contains design values for timbers with uniform grade lay-ups. These combinations are intended primarily for timbers loaded axially or in bending due to loads applied parallel to the wide faces of the laminations <u>about the y-y axis.</u> Design values are included, however, for bending due to loads applied perpendicular to the wide face of the laminations <u>about the x-x axis.</u></p>	Accept change and re-circulate as substantive.

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 15 3.1	Marc Mullins	The wording used in 3.1 should be consistent with Sections 4.3.3.1 and 4.3.3.2 of ANSI/AITC A190.1-2007	Lumber grades shall be in accordance with Section 4.3 – <i>Lumber for Laminating of ANSI/AITC A190.1. AITC Grading Handbook for Laminating Lumber</i> (1) summarizes the requirements for laminating grades of approved species and references approved grading rules.	Lumber used for laminations shall be graded according to the standard grading rules approved by the Board of Review of the American Lumber Standard Committee and/or special laminating rules.	No action recommended; ANSI/AITC A190.1-2007, Section 4.3 includes material that may be used for laminating that does not pertain to grading rules approved by the Board of Review of the American Lumber Standard Committee and/or special laminating rules.
Item 16 3.1	Kevin Cheung	The 2008 edition of the AITC Grading Handbook for Laminating Lumber deviates from the grading rules for Structural Laminations in the Standard Grading Rules for Western Lumber as follows: - Twist provision is not included in AITC Grading Handbook - “Timber Break” described in the AITC Grading Handbook is inconsistent/incorrect. ALSC’s NGR Interpretation provides the proper description of “Timber Break”. - The Slope of Grain provision in the AITC Grading Handbook Table 3.5.2 for Hem-Fir L1 and LID grades are different from the Standard Grading Rules for Western Lumber.	Lumber grades shall be in accordance with Section 4.3 – <i>Lumber for Laminating of ANSI/AITC A190.1. AITC Grading Handbook for Laminating Lumber</i> (1) summarizes the requirements for laminating grades of approved species and references approved grading rules.	The 2008 edition of the AITC Grading Handbook for Laminating Lumber deviates from the grading rules for Structural Laminations in the Standard Grading Rules for Western Lumber as follows: - Twist provision is not included in AITC Grading Handbook - “Timber Break” described in the AITC Grading Handbook is inconsistent/incorrect. ALSC’s NGR Interpretation provides the proper description of “Timber Break”. - The Slope of Grain provision in the AITC Grading Handbook Table 3.5.2 for Hem-Fir L1 and LID grades are different from the Standard Grading Rules for Western Lumber.	No action recommended. See Item 15.

Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 17 3.3	Rick Story	I suggest the following calculations instead of those in the preceding 2 sentences: The number of L1 laminations is : $16(0.05 + 0.15) - 1 = 2.2$ lams (round up to 3). The number of L2 lams is: $16(0.05 + 0.15 + 0.10) - (1 + 3) = 4.8 - 4 = 0.8$ (round up to 1).	“The tension zone of a hypothetical 16 lamination beam requires 5% 302-24, 15% L1, and 10% L2. The number of 302-24 laminations is determined by: $16 \times 0.05 = 0.8$ (rounded up to 1). The number of L1 laminations is: $16 \times 0.15 - (1 - 0.8) = 2.4 - 0.2 = 2.2$ lams (round up to 3). The number of L2 lams is $16 \times 0.1 - (3 - 2.2) = 1.6 - 0.8 = 0.8$ (rounded up to 1).”	I suggest the following calculations instead of those in the preceding 2 sentences: The number of L1 laminations is : $16(0.05 + 0.15) - 1 = 2.2$ lams (round up to 3). The number of L2 lams is: $16(0.05 + 0.15 + 0.10) - (1 + 3) = 4.8 - 4 = 0.8$ (round up to 1).	No action recommended.
Item 18 3.3	Rick Story	Least 2 – 1 3/8” lams = 2 3/4”	... then a total thickness of at least 2-3/4 in. (70 mm) of L1 grade is required in that zone.	Least 2 – 1 3/8” lams = 2 3/4”	No action recommended.
Item 19 3.4	Marc Mullins	The use of “Appearance Grades” may be confusing with the mechanical property requirements.	... members shall be for industrial or framing appearance grades and for prismatic members only.	Suggest changing it to “Appearance Classifications,” as used by APA Y110.	No action recommended. See Item 6.
Item 20 3.7	Jeff Linville	ANSI/AITC A190.1 already requires “TOP” to be marked on all beams. Do we need to repeat that requirement for this particular case?	Lay-ups modified to meet these requirements shall be marked with “1-HOUR FIRE RATING” if one additional tension lamination is used or “2-HOUR FIRE RATING” if two additional tension laminations are used. Additionally, balanced lay-ups designed and manufactured for three-sided fire exposure shall be marked with “TOP” on the appropriate face to ensure proper orientation in the structure.	Lay-ups modified to meet these requirements shall be marked with “1-HOUR FIRE RATING” if one additional tension lamination is used or “2-HOUR FIRE RATING” if two additional tension laminations are used. Additionally, balanced lay-ups designed and manufactured for three-sided fire exposure shall be marked with “TOP” on the appropriate face to ensure proper orientation in the structure.	Accept change and re-circulate as substantive.

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 21 3.8	Marc Mullins	This section shall permit the option that the design value is adjusted in accordance with Table A3.	Where necessary, core laminations shall be removed and additional compression laminations shall be added to meet this requirement.	Where necessary, core laminations shall be removed and additional compression laminations shall be added to meet this requirement. <u>Otherwise, the beam design values shall be adjusted in accordance with Table A3.</u>	No action recommended. Table A3 addresses Taper Cuts; Section 3.8 addresses Bevel Cuts.
Item 22 All Tables	Jeff Linville	Do we need to create metric versions of the tables? Can we just include a footnote with the conversion factor? If we have to convert the table values do we create duplicate tables or place the metric values in parentheses alongside the inch-pound values?		Place metric conversion factor in a footnote in the table (first preference) If a footnote isn't adequate, create separate metric versions of the larger tables. The smaller tables could possibly just have metric units added after the inch-pound units.	Recommend editorially adding additional metric versions of larger tables to ensure compliance with ANSI approved procedures.
Item 23 Table A1 and A1 Expanded Vote	Jeff Linville	The headings for the flexure values should be changed to reflect the text in 2.2, which states, "Positive bending causes tensile stresses at the bottom of the beam. Negative bending causes compressive stresses at the bottom of the beam."	"Tension zone stressed in tension." "Compression zone stressed in tension"	Revise Table Headings: " Tension zone <u>Bottom of beam</u> stressed in tension (<u>positive bending</u>)" " Compression zone <u>Top of beam</u> stressed in tension (<u>negative bending</u>)"	Accept change and re-circulate as substantive.

Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 24 Table A1	Marc Mullins	Footnote 8 to Table A1 is inconsistent with Footnote 6 to Table A1-Expanded. There is no reason these 2 footnotes should be different. Footnote 6 to Table A1-Expanded reflects more accurately on the current industry practice.	Table A1, Footnote 8: 30F combinations are restricted to a maximum 6 in. nominal width. Table A1-Expanded, Footnote 6: 30F combinations are restricted to a maximum 6 in. nominal width unless the manufacturer has qualified for wider widths based on full-scale tests subject to approval by an accredited inspection agency	Replace Footnote 8 to Table A1 by Footnote 6 to Table A1-Expanded.	Accept changes as editorial.
Item 25 Table A3	Marc Mullins	It is unclear why the reduced design values for the stress classes are so conservative, as compared to the reduced design values for individual layup combinations within the stress class. For example the reduced Fbx + for 16F-1.3E is tabulated at 1,050 psi. However, the minimum Fbx + for all layup combinations within this stress class is 1,350 psi.		Suggest the reduced values for the stress classes be doublechecked.	No action recommended.
Item 26 Appendix B, Table B1	Jon Howard	There is no reference to a 6" nominal width limitation for 30F-E1 SP and 30FE2 Sp		Include a footnote with the same wording as footnote 6 for Table A1-Expanded.	Accept changes as editorial.

Item No. Section	Received from	Explanation	Original Wording	Proposed wording or Comment	Recommended Action
Item 27 Appendix B	Marc Mullins	The width limitation of 6 inches or less on 30F-E1/SP and 30F-E2/SP is not included in the layup table. Even though Footnote 6 to Table A1-Expanded has such a limitation, this could be easily overlooked.		Include a footnote with the same wording as footnote 6 for Table A1- Expanded.	Accept changes as editorial.

Negative

Panel of Directors of the AITC Standards Committee
Disposition of Negative Ballots

Item 1

Voter's Name: Doug Hucke

Affiliation: Timberweld
Manufacturing

Section: 1.8

Current Draft Text:

Structural glued laminated timbers can be manufactured in a variety of shapes from straight beams to curved arches. Members can also be manufactured with tapered or constant cross section.

For curved members of Southern Pine, the minimum radius (at the inside face of curvature) shall not be less than 100 times the lamination thickness. For curved members of other softwood species, the minimum radius shall not be less than 125 times the lamination thickness.

Explanation:

The 1/100 & 1/125 radius criteria are unconservative in some cases. Members configured at the minimum radii may exhibit excessive spring back or splitting on the ends and outside face. The 117 standard should guide the user to commonly used standard radii

Proposed revision:

Restore the previous language:

“The recommended minimum radii

7 ft 0 in

9 ft 4 in

For less sharply ...

27 ft 6 in”

Recommended Action:

Revise text to read:

1.8. Shapes

Structural glued laminated timbers can be manufactured in a variety of shapes from straight beams to curved arches. Members can also be manufactured with tapered or constant cross section.

1.8.1. For curved members manufactured with nominal 2 in. thickness laminations, the minimum radius of curvature (at the inside face) is typically 27 ft 6 in. for all softwood species.

1.8.2. For tudor arches and other tightly curved members manufactured with nominal 1 in. thickness laminations, typical minimum radii of curvature (at the inside face) are:

7 ft 0 in.* for Southern Pine

9 ft 4 in.* for all other softwood species

Panel of Directors of the AITC Standards Committee
Disposition of Negative Ballots

* The manufacture of curved members with radii shorter than these typically requires standard thickness laminations to be planed to a thinner dimension resulting in more waste and less efficient use of materials. It is recommended that the designer contact the laminator prior to specifying radii shorter than those listed above. For thin laminations, the radius shall not be less than 100 times the lamination thickness for Southern Pine or 125 times the lamination thickness for other softwoods.

Action:

Revise text:

Non-persuasive, no change

Reason:

Panel of Directors of the AITC Standards Committee
Disposition of Negative Ballots

Item 2

Voter's Name: Don DeVisser

Affiliation: WCLIB

Section: 3.1

Current Draft Text:

Lumber grades shall be in accordance with Section 4.3 – *Lumber for Laminating* of ANSI/AITC A190.1. *AITC Grading Handbook for Laminating Lumber* (1) summarizes the requirements for laminating grades of approved species and references approved grading rules.

Explanation:

The proposed revision is very similar in wording in ANSI/AITC A190.1-2007, sections 4.3.3.1 and 4.3.3.2. This will help make the two standards more consistent.

Proposed revision:

Lumber used for laminations shall be graded according to the standard grading rules approved by the Board of Review of the American Lumber Standard Committee and/or special laminating rules.

Recommended Action:

No change recommended. ANSI/AITC A190.1-2007, Section 4.3 includes material that may be used for laminating that does not pertain to grading rules approved by the Board of Review of the American Lumber Standard Committee and/or special laminating rules.

Action:

Revise text:

Non-persuasive, no change

Reason: